

## CHAPTER-17

### DAM BREAK ANALYSIS AND DISASTER MANAGEMENT PLAN

#### 17.1. NEED FOR DAM BREAK STUDY

Any hydroelectric project if not designed on sound principles of design after detail investigations in respect of hydrology, geology, seismicity etc., could result in a large scale calamity. Thus there are inherent risks to the project like improper investigation, planning, designing and construction which can ultimately lead to catastrophic situation. Though through detailed field investigations it has been ensured that the dam is founded on firm foundation, designed for suitable seismic design parameters and the spillway has been designed for passing Standard Project Flood (SPF) 4245 cumecs discharge, yet in view of that uncertain element of “Force Mejure” the eventuality of a disaster cannot be ignored but a rescue plan has to be devised for implementation during such an exigency without being caught in the vast realm of unpreparedness.

A disaster is an unwarranted, untoward and emergent situation that culminates into heavy toll of life and property and is a calamity sometimes caused by “force majeure” and also by human error. The identification of all types of disaster in any proposed project scenario involves the critical review of the project vis-à-vis the study of historical past incidents/disasters in the similar situations. The evolution of disaster management plan dwells on various aspects such as provision of evacuation paths, setting up of alarms and warning systems, establishing communicating system besides delineating an Emergency Response Organization with an Effective Response System. Keeping in view the grievous affects a disaster can cause on human or animal population, loss of property and environment in and around the

areas of impact. Therefore it is essential to assess the possibility of such failures in context to the present project and formulate a contingent plan.

### 17.2. MODEL FOR DAM BREAK ANALYSIS

For reasons of simplicity, generally, wide applicability and the uncertainty in the actual mechanism, the **BOSS DAMBRK** model has been used. The model uses failure time interval, terminal size and shape of the breach as the inputs. The possible shapes of the breach that can be accomplished by the model are rectangular, triangular and trapezoidal. The model is capable of adopting either storage routing or dynamic routing methods for routing floods through reservoirs depending on the nature of flood wave movement in reservoirs at the time failure.

The dynamic routing method based on the complete equations of unsteady flow is the appropriate technique to route the flood hydrograph through the downstream valley. The method is derived from the original equations developed by St. Venant. The model uses St. Venant's equations for routing dam break floods in channels.

### 17.3. METHODOLOGY

The National Weather Service's DAMBRK model developed by Dr. L. Fread has been used in the study. This model simulates the failure of dam, computes the resultant outflow hydrograph and also simulates movement of the dam break flood wave through the downstream river valley. The model is built around three major capabilities, which are reservoir routing, breach simulation and river routing. However, it does no rainfall-runoff analysis and storm inflow hydrographs to the upstream of reservoir must be developed external to the model. A brief description of the capabilities of the model is described in the following paragraphs

### 17.3.1 Reservoir Routing

The storage routing is based on the law of conservation given as:

$$I - Q = dS/dt \dots\dots\dots (1)$$

in which, I is reservoir inflow. Q is the total reservoir outflow which includes the flow spillway, breach, overtopping flow and head independent discharge, and rate of change of reservoir storage volume. Equation (1) can be expressed in finite difference form as :

$$(I + I')/2 - (Q + Q')/2 = \Delta S/\Delta t \dots\dots\dots (2)$$

in which the prime ( ' ) superscript denotes the values at the time  $t - \Delta t$  and the notation approximates the differential. The term  $\Delta S$  may be expressed as:

$$\Delta S = (A_s + A'_s) (h - h')/2 \dots\dots\dots (3)$$

in which,  $A_s$  is the reservoir surface area coincidental with the elevation (h) and is a function of h. The discharge Q which is to be evaluated from equation (2) is a function of h and this known h is evaluated using Newton–Raphson iteration technique and thus the estimation of discharge corresponding to h.

### 17.3.2 Dynamic Routing

The hydrologic storage routing technique, expressed by equation (2) implies that the water surface elevation within the reservoir is horizontal. This assumption is quite adequate for gradually occurring breaches with no substantial reservoir inflow hydrographs. However, when the breach is specified to form almost instantaneously so as to produce a negative wave within the reservoir, and/or the reservoir inflow hydrograph is significant enough to produce a positive wave progressing through the reservoir, a routing option which simulates the negative and /or positive wave occurring within the reservoir may be used in DAMBRK model. Such a technique is referred to as dynamic routing. The routing principle is same as dynamic routing in

river reaches and it is performed using St. Venant's equation. The movement of the dam break flood wave through the downstream river channel is simulated using the complete unsteady flow equations for one dimensional open channel flow, alternatively known as St. Venant's equations. These equations consist of the continuity equation

$$\frac{\partial Q}{\partial t} + \frac{\partial(A + A_0)}{\partial t} = q \dots\dots\dots(4)$$

and the conservation of momentum equation :

$$\frac{\partial Q}{\partial t} + \frac{\partial(A_2/A + A)}{\partial t} + g A \left( \frac{\partial h}{\partial t} + S_f + S_e \right) + L_c = 0 \dots\dots(5)$$

where,

A = active cross – sectional flow area

A<sub>0</sub> = inactive (off-channel storage) cross – sectional area

X = distance the channel

q = lateral inflow or outflow per unit distance along the channel

g = acceleration due to gravity

Q = discharge

H = water surface elevation

S<sub>s</sub> = friction slope

S<sub>e</sub> = expansion – contraction loss slope

L<sub>c</sub> = lateral inflow/outflow momentum effect due to assumed flow path of inflow being perpendicular to the main flow

The friction slope and expansion – contraction loss slope are evaluated by the following equation

$$S_f = \frac{n^3 Q^2}{2.21 A^2 R^{3/4}} \dots\dots\dots(6)$$

and,

$$S_e = \frac{K \Delta(Q/A)^2}{2g \Delta X} \dots\dots\dots(7)$$

where,

n = Manning's roughness coefficient

R = A/B where B is the top width of the active portion of the channel

K = Expansion – contraction coefficient varying from 0.1 to 0.3 for contraction and 0.5 to – 1.0 expansion

$\Delta(Q/A)^2$  = Difference in (Q/A)<sup>2</sup> for cross sections at their end of a reach

The non-linear partial differential equations (4) and (5) are represented by a corresponding set of non-linear finite difference algebraic equations and they are solved by the Newton – Raphson method using weighted four point implicit scheme to evaluate  $Q$  and  $h$ . The initial conditions are given by known steady discharge at the dam, for which steady state nonuniform boundary flow equation are used. The outflow hydrograph from the reservoir is the upstream boundary condition for the channel routing and the model is capable of dealing with fully supercritical flow or fully supercritical flow in the reach or the upstream reach having supercritical flow and downstream reach having sub critical flow. There is a choice of downstream boundary conditions such as internally calculated loop rating curve, user provided single valued rating curve, user provided time dependent water surface elevation, critical depth and dam which may pass flow via spillways, overtopping and/or breaching.

### **17.3.3 Statement of the problem**

The computation of flood wave resulting from a dam breach basically involves two scenarios which can be considered jointly or separately: (1) the outflow hydrograph from the pond (2) the routing of the flood wave downstream from the breached dam along the river valley and the flood plain. If breach outflow is independent of downstream conditions, or if their effect can be neglected, the reservoir outflow hydrograph is referred to as the free outflow hydrograph. In this case, the computation of the flood characteristics is divided into two distinct phases: (a) the determination of outflow hydrograph with or without the routing of the negative wave the reservoir, and (b) the routing of flood wave downstream from the breached dam. In this study the problem of simulating the failure of “Dam” and computing the free outflow hydrograph from the breached section using storage routing technique’ with the aim of

reproducing the maximum water level marks reached during the passage of flood wave is considered. The information regarding inflow hydrograph into the pond due to the storm at the time of failure, the structural and the hydraulic characteristics details of the dam, the time of failure, the channel cross sections details, the maximum water level marks reached in the reservoir at the time of failure and those observed in the downstream reach of the dam to the passage of flood wave etc. are available for the study.

#### **17.3.4 Availability of Data**

The input data required for the National Weather Service's BOSS DAMBRK model can be categorized into two groups. The first data group pertains to the dam and inflow hydrograph into the reservoir and the second group pertains to the routing of the outflow hydrograph through the downstream valley. These are described in the following paragraphs.

- **First Data Group**

With reference to the data group pertaining to the dam, the information on reservoir elevation-volume relationship, spillway details, elevation of bottom and top of dam, elevation of water surface in the pond at the beginning of analysis and at the time of failure, breach description data are required. The particulars of the data used under each of the above mentioned categorized are given in Annexure I.

- **Second Data Group**

The second group of data pertaining to the routing of the outflow hydrograph through the downstream valley consists of a description of cross-sections, hydraulic resistance coefficients of the reach, steady state flow in the river at the beginning of the simulation and downstream boundary condition. The cross section is specified by

location mileage, and tables of top width and corresponding elevation. In this study, seven cross sections details, at km. 0.0, 1.0, 5.0, 12.0, 18.0, 22.0 and 25.0 km downstream of dam, have been used.

#### 17.4 RESULT AND CONCLUSIONS

A rectangular breach with bottom width of 45 m. at an El 1268 masl with side slope 1:0 and breach formation time as 0.5 hr. have been considered in the study. After the breach, immediately below the dam, the maximum flow will occur immediately after the start of breach. The magnitude of the simulated outflow hydrograph will be 11114 cumec corresponding to maximum stage elevation 1303.07 masl at Km. 0.00 is attenuated to 10677 cumecs corresponding to maximum stage elevation of 1010.30 masl at km. 25.0. The inflow hydrograph when propagated through the valley, without dam break, is not substantially attenuated. The combined outflow hydrograph from dam break and without dam break and the maximum stage elevation curve due to dam break have been presented in separate graphs enclosed with Attachment -I.

The maximum flow and maximum velocity at various distances d/s of the dam is shown in following Table-17.1.

**TABLE-17.1**

**Summary of wave profile in the event of Dam Break**

<b>Distance from Dam (km)</b>	<b>Max Elevation, (masl)</b>	<b>Maximum Flow (cumec)</b>	<b>Time to Maximum Stage</b>	<b>Maximum Velocity (m/s)</b>
0.000	1303.07	11114	0.200	7.73
1.000	1270.11	11114	0.225	10.26
2.000	1259.00	11101	0.250	9.44
3.000	1248.07	11086	0.275	8.89
4.000	1237.27	11069	0.300	8.48
5.000	1126.48	11050	0.325	8.25
6.000	1215.43	11032	0.350	8.22
7.000	1204.45	11014	0.375	8.16
8.000	1193.54	10997	0.400	8.09
9.000	1182.68	10982	0.425	8.03

Distance from Dam (km)	Max Elevation, (masl)	Maximum Flow (cumec)	Time to Maximum Stage	Maximum Velocity (m/s)
10.000	1171.89	10966	0.475	7.95
12.000	1150.57	10929	0.500	7.66
18.000	1089.94	10864	0.700	6.47
22.000	1053.07	10714	0.750	10.82
25.000	1010.30	10677	0.825	9.87

The following conclusions could be drawn from **Table-17.1**:

- Failure of concrete dam like the proposed Devsari Hydroelectric Project, which is designed to the present technical standards and built with adequate quality control, is a very remote possibility.
- The monoliths having the least resistance to withstand the unforeseen loading combinations may give way, which in turn provides a relief and prevents failure of other monoliths. Under such as situation, the discharge and the water depth will be much lesser than those determined from the study.

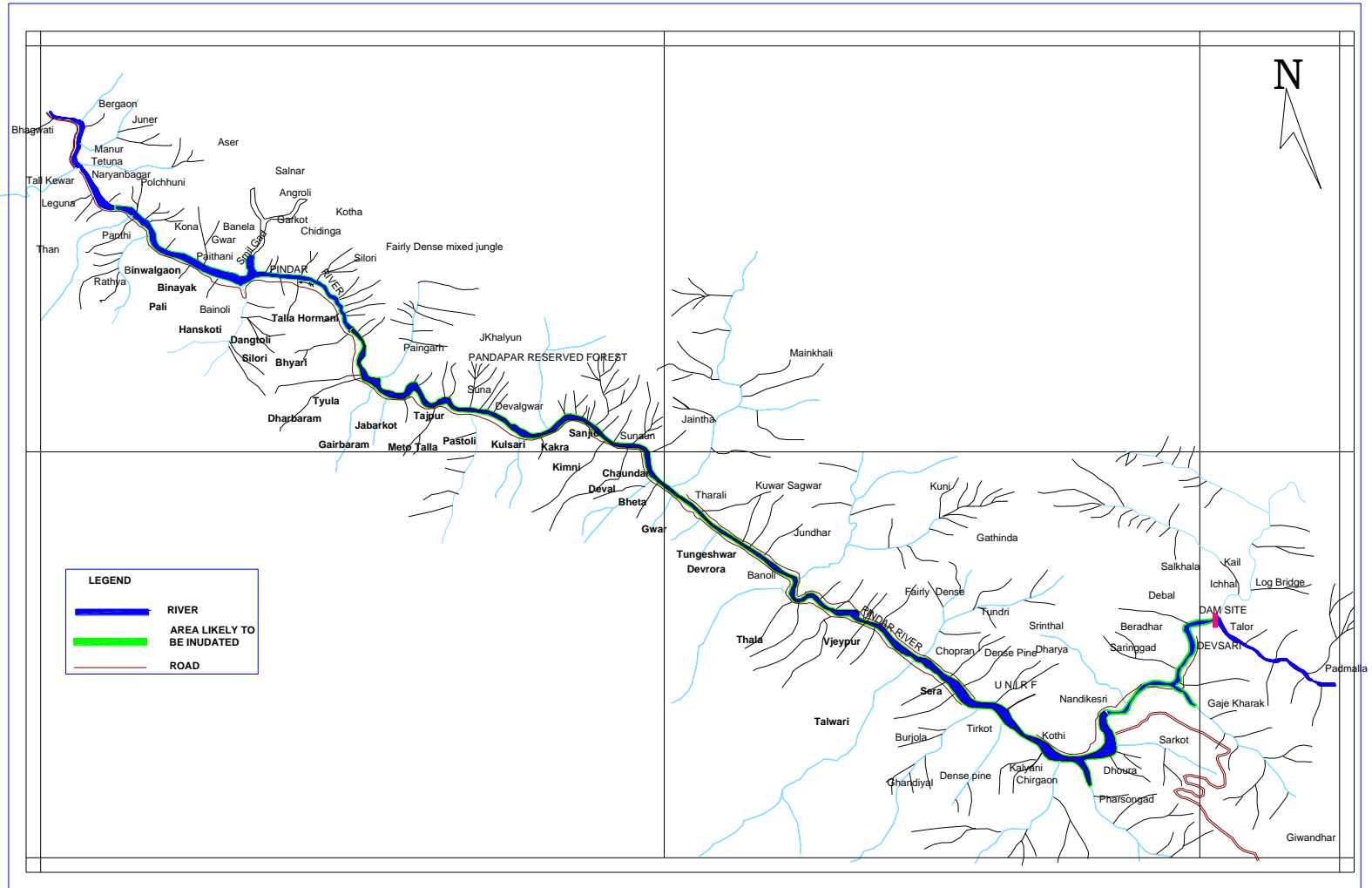
## 17.5 DISASTER MANAGEMENT PLAN

The emergency planning for dam break scenario is devised on the basis of results of dam break analysis mainly the travel time of flood wave to various locations in the downstream stretch of the river. It is inferred from the analysis that in case of main dam failure the flood peak discharge as it propagates through the valley on d/s shall partly affect eleven (11) and seven (07) settlements on left and right bank respectively. The flood wave shall reach twenty-two km. in 45 minutes. However, the flood wave shall also inundate some portion of Narain Bagar – Tharali Road at such stretches which are very close to the river. In view of the fact that even upto 25 km d/s of the dam upto Narain Bagar, settlements very near to the river shall be partially affected, a disaster management plan is warranted. Besides this due to the breaking of the dam the major risk shall be to the traffic on Narain Bagar – Tharali road and to the persons



who have for some reason or the other have entered the river bed. The plan is, therefore, based on such measures, which are purely preventive in nature.

The flood period during monsoon generally is reckoned from June with the onset of monsoon and ends with withdrawal of south-west monsoon by the end of September. Before the onset of monsoon all hydro-mechanical equipments, electrical gadgets, captive power plant and public announcement and communication system should be kept in perfect readiness. The degree of alertness has to enhance during high stage of river manifested with sharp increase in discharge. Though there can not be very sharp edge demarcation between different levels of emergency yet the following flood conditions have been contemplated and the preventive measures suggested against each in the following **Table-17.2**. The inundation map is enclosed as **Figure -17.1**



### Figure-17.1: Inundation Map

**TABLE-17.2****Preventive measures for various levels of emergency**

<b>S. No.</b>	<b>Level of emergency</b>	<b>Water Level</b>	<b>Preventive measures</b>
1.	Normal Flood	Below FRL i.e. EL 1300 masl	Utmost vigil observed in regulation of spillway gates
2.	Level –1 Emergency	Rises above EL 1300 masl but below EL 1303 masl	(1) All gates fully operational (2) All the officials should attend dam site. Local officials informed and warning system be kept on alert. Communication and public announcement system should be put into operation and flood warning issued to people
3.	Level –2 Emergency	Top of dam i.e. EL 1303 masl	(1) All staff from dam site, power house and TRT outlets alerted to move to safer places (2) Possibility of dam failure should be flashed to District Administration.
4.	Disaster	Rising above EL 1303 masl and the breach appears in any form	District Administration and Project authorities be intimated and only life saving measures should be resorted too

**17.5.1 Dam Safety and Maintenance Manual**

Based on standard recommended guidelines for the safety inspection of dams a manual should be prepared by the project proponents in respect of dam safety surveillance and monitoring aspects. This should be updated with the availability of instrumentation data and observation data with periodical review. The need for greater vigil has to be emphasized during first reservoir impoundment and first few years of operation. The manual should also delve on the routine maintenance schedule of all hydro-mechanical and electrical instruments. It should be eloquent in respect of quantum of specific construction material needed for emergency repair along with

delineation of the suitable locations for its stocking and also identify the much needed machinery and equipment for executing emergency repair work and for accomplishing the evacuation plan.

#### **17.5.2 Emergency Action Plan (EAP)**

Dam safety programme as indicated above includes the formation of an Emergency Action Plan for the dam. An emergency is defined as a condition of serious nature which develops unexpectedly and endangers downstream property and human life and required immediate attention. Emergency Action Plan should include all potential indicators of likely failure of the dam, since the primary concern is for timely and reliable identification and evaluation of existing of potential emergency.

This EAP presents warning and notification procedures to follow during the monsoon season in case of failure or potential failure of the dam. The objective is to provide timely warning to nearby residents and alert key personnel responsible for taking action in case of emergency.

#### **17.5.3 Administration and Procedural Aspects**

The administrative and procedural aspects of the Emergency Action Plan consist of flow chart depicting the names and addresses of the responsible personnel of SJVN Ltd. and the Dist. Administration, Chamoli. In order of hierarchy, the following system will usually be appropriate. In the event that the failure is imminent or the failure has occurred or a potential emergency conditions is developing, the observer at the site is required to report it to the Junior Engineer / Assistant Engineer of SJVN Ltd. who will report to the Executive Engineer / Superintending Engineer for their reporting to the Chief Engineer through a wireless system or by any available fastest communication system. The Engineer-in-Charge is usually responsible for making cognizant with the developing situation to the Civil Administration viz. District Magistrate, Chamoli and

concerned sub-divisional magistrates and tehsildars. Each personnel is to acknowledge his/her responsibilities under the EAP in an appropriate format at a priority.

The technical aspects of the EAP consist of preventive action to be taken with regards to the structural safety of the dam. The EAP is drawn at a priority for the regular inspection of the dam. For this purpose, providing an adequate and easy access to the dam site is a necessity. The dam, its sluices, overflows and non-overflow sections should be properly illuminated for effective operations during night time. Whenever sinkholes, boils, increased leakages, movement of masonry rock, gate failure, rapid rise or fall of the level in the reservoir, rise in the level of reservoir beyond the maximum working level, or wave overrun of the dam crest are observed, the personnel on patrol is required to inform immediately to the Junior Engineer (JE) / Assistant Engineer (AE) for initiation of the execution of EAP. They are required to inform the Engineer-in-Charge and the local administrative authorities. It is desirable if the downstream inhabitants are warned using siren, if available, so as to make them aware the likely imminent danger.

The other preventive measures may include availability of sufficient number of sandbags at several selected downstream locations and logs (for holding sandbags) and at the dam site, one tractor, two motor boats, gas lanterns, Manila ropes and life jackets. Areas from where the labour can be mobilized should be chalked out at a priority. In addition to these, public participation in the process of execution of the EAP may further help in amelioration of the adverse impacts of the likely disaster. For this, it is necessary that the public should be made aware of its responsibilities. The flow chart of the responsibilities of SJVN Ltd. and Civil Administration, Chamoli is shown in

**Figure-17.2**

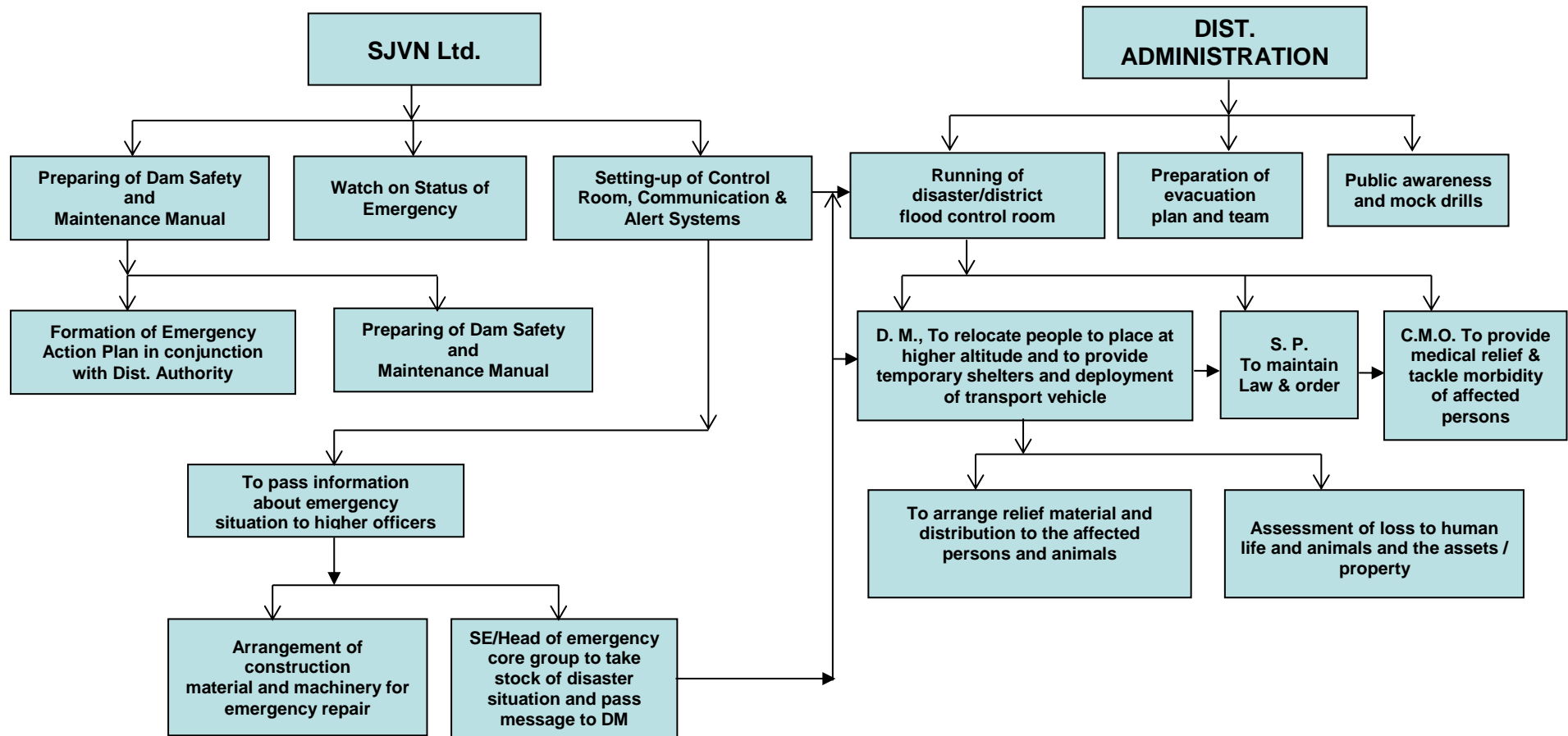


Figure-17.2: Flow Chart of Responsibilities

#### **17.5.4 Preventive Action**

Once the likelihood of an emergency situation is suspected, action has to be initiated to prevent a failure. The point at which each situation reaches an emergency status shall be specified and at that stage the vigilance and surveillance shall be upgraded both in respect of time and level. At this stage a thorough inspection of the dam should be carried out to locate any visible sign(s) of distress.

Engineers responsible for preventive action should identify sources of equipment needed for repair, materials, labour and expertise for use during an emergency. The amount and type of material required for emergency repairs should be determined for dam, depending upon its characteristics, design, construction history and past behavior. It is desirable to stockpile suitable construction materials at appropriate sites. The anticipated need of equipment should be evaluated and if these are not available at the dam site, the exact location and availability of these equipments should be determined and specified. The sources/agencies must have necessary instructions for assistance during emergency. Due to the inherent uncertainties about their effectiveness, preventive actions should usually be carried out simultaneously with the appropriate notification on alert situation or a warning situation.

#### **17.5.5 Communication System**

An effective communication system and a downstream warning system are absolutely essential for the success of an emergency preparedness plan. The

difference between a high flood and dam-break situation must be made clear to the downstream population.

#### **17.5.6 Evacuations Plans**

Emergency Action Plan includes evacuation plans and procedures for implementation based on local needs. These could be:

- Demarcation / prioritization of areas to be evacuated.
- Notification procedures and evacuation instructions.
- Safe routes, transport and traffic control.
- Safe areas/shelters.
- Functions and responsibilities of members of evacuation team.

Any precarious situation during floods will be communicated either by an alert situation or by an alert situation followed by a warning situation. An alert situation would indicate that although failure of flooding is not imminent, a more serious situation could occur unless conditions improve. A warning situation would indicate that flooding is imminent as a result of an impending failure of the dam. It would normally include an order for evacuation of delineated inundation areas.

#### **17.5.7 Evacuation Team**

It will comprise of following official / Representative:

- District Magistrate (D. M.), Chamoli / his Nominated officer (To peacefully relocate the people to places at higher elevation with state administration).
- Engineer in charge of the project (Team Leader)
- Superintendent of Police (S. P.), Chamoli / Nominated Police Officer (To maintain law and order)
- Chief Medical Officer (C. M. O.), Chamoli (To tackle morbidity of affected people)
- Sarpanch/Affected village Representative to execute the resettlement operation with the aid of state machinery and project proponents.
- Sub committees at village level

The Engineer-in-Charge will be responsible for the entire operation including prompt determination of the flood situation time to time. Once the red alert is



declared the whole state machinery will come into swing and will start evacuating people in the inundation areas delineated in the inundation maps. For successful execution, annually demo exercise will be done. The District Magistrate is to monitor the entire operation.

#### **17.5.8 Public Awareness for Disaster Mitigation**

In addition, guidelines that have to be followed by the inhabitants of flood prone areas, in the event of flood resulting from dam failure, which form part of public awareness for disaster mitigation may also include following:

- Listen to the radio for advance information and advice.
- Disconnect all electrical appliances and move all valuable personal and household goods beyond the reach of floodwater, if one is warned or if one suspects that flood waters may enter the house.
- Move vehicles, farm animals and movables goods to the higher place nearby.
- Keep sources of water pollution i.e. insecticides out of the reach of water.
- Turn off electricity and LPG gas before one has to leave the house.
- Lock all outside doors and windows if one has to leave the house.
- Do not enter floodwaters.
- Never wander around a flood area.

#### **17.5.9 Notifications**

Notification procedures are an integral part of any emergency action plan. Separate procedures should be established for slowly and rapidly developing situations and failure. Notifications would include communication of either an alert situation or an alert situation followed by a warning situation. An alert situation would indicate that although failure or flooding is not imminent, a more serious situation could occur unless conditions improve. A warning situation would indicate that flooding is imminent as a result of an impending failure of the dam. It would normally include an order for evacuation of delineated inundation areas.

#### **17.5.10 Notification Procedures**

Copies of the EAP that also include the above described inundation map are displayed at prominent locations, in the rooms and locations of the personnel named in the notification chart. For a regular watch on the flood level situation, it is necessary that the flood cells be manned by two or more people so that an alternative person is always available for notification round the clock. For speedy and unhindered communication, a wireless system is a preferable mode of communication. Telephones may be kept for back up, wherever available. It is also preferred that the entire flood cells, if more than one, are tuned in the same wireless channel. It will ensure communication from the dam site to the control rooms. The communication can be established by messenger service in the absence of such modes of communication.

#### **17.5.11 Management after receding of Flood Water**

It is to be accepted that in the event of dam break, even with maximum efforts, the loss of human lives, livestock and property would be inevitable. Under such a scenario, a massive effort would be used by various government agencies to provide various relief measures to the evacuees. Formulation of a plan delineating such measures is beyond the scope of work of this document. However, some of the measures which need to be implemented are listed as below :

- Provision of various food items and shelter to the evacuees.
- Provision of fuel for various evacuees.
- Provision of adequate fodder supply.
- Arrangements for potable water supply.

- Commissioning of low cost sewage treatment and sanitation facilities, and disposal of treatment sewage.
- Expeditious disposal of dead bodies human and livestock.
- Immunization programmes for prevention of outbreak of epidemics of various water related diseases.
- Adequate stocks of medicines of various diseases, especially water-related diseases.

#### **17.5.12 Communication between Different Projects**

The Pinder river meets the Alaknanda at Karanpryag in district Chamoli. Shrinagar Hydel Scheme on river Alaknanda, near Shrinagar town in Pauri Garhwal is under construction. Besides this, Kotlibhel HEP (Stage-IB) has also been proposed on river Alaknanda. These projects shall run in tandem. Therefore, a comprehensive monitoring network system has to be established to jointly combat the threat due to failure of any of these schemes. For accomplishing these, a full proof “no communication failure” system has to be evolved.

#### **17.6 FLOOD FORECASTING**

The importance of flood forecasting is paramount in a dam break scenario, by overtopping, when little or no reaction time is left for the people to evacuate to safe places. Effective and accurate flood warning can facilitate the evacuation of people living in flood zone, their property and livestock, opportune maintenance and early alerting of emergency services besides exercising legitimate control by adjusting downstream releases from reservoir / ponds or achieving the balance pond. An advance warning of approaching flood allows suitable reservoir operation for moderating its intensity / peak and also helps in ensuring full storage and for flood relief purposes.

The catchment of the Pinder, a tributary of Alaknanda, located in Mid Himalayas, upto the proposed dam site, extends approximately 1138 Sq Km in area. River flows are constituted of two main natural components viz., run-off resulting from precipitation and base flow derived from spring flows. Due to mountains topography, the excessive bed slope causes rapid run-off from the contributing hill torrents to the main river. There is no extensive network of meteorological station, rain gauge; snow-gauge; gauge and discharge sites in the catchment area. Many of the sub-watersheds within the catchment are un-gauged. Currently flood warning relies on issuing of alerts when the river level at a few location reaches are within a few meter below the high flood levels observed in the past. Sometime these warnings may be accurate but due to very little lead time between the HFL being very fast approached and the commencement of flooding. Due to the existence of fertile agricultural land and its expansion along the river banks and concentration of population in the region of submergence area, there is a need for developing an operational flood forecasting system as a part of preparedness strategies for disastrous flood events by providing advance warning several days ahead such that the public and the district authorities have adequate time at their disposal without being panicky.

Due to morphological characteristics, the flood plains and the area near to the river/stream banks, classified under land use class agriculture and settlement, are more prone to the flood hazards. In such areas delineation of flood zone and its height besides detecting the characteristics of floods in different return periods is most significant. Thus flood zonation is not only essential in respect of various

development activities in the likely inundation area, but also for study of ecological and environment impacts. For the study of flood zonation, within the likely inundation area, for different time periods of 2, 5, 10, 25, 50 and 100 years, topography maps at 1:1000 scale shall have to be developed.

All forms of flood forecasting use some type of trigger mechanism to anticipate when the water level of the river at the flood risk area shall exceed the threshold. When the trigger reaches a predetermined level that is less than the threshold, a warning is triggered. In case of a small river, a rainfall-runoff based model may be adequate within reasonable limits of accuracy. In case of large rivers, like Pinder, forecasting of discharge by upstream stages, with a high degree of sophistication is involved. The main aim is to assess the future output at different time as accurately as possible, i.e. within narrow error bonds, starting with measurements of present and past input quantities. Interaction between a comprehensive hydrological model and geographical information system (GIS) technique provides a better forecasting tool. The main requirement of a hydrological model is description of flow channel characteristics and land surface as input data to the watershed model. The flood zonation is actualize, development and perfection of the applied engineering hydrology and its aim is to acquire a real time rainfall data and river flow by short wave, radio and satellite network, and using them tin rainfall runoff models to forecast.

For enabling GIS based flood forecasting using hydrological model, a network of automatic meteorological station, rainfall gauges, gauge and discharge sites

equipped with latest state-of-the-art gadgets, meteorological radar shall have to be established.

### 17.7. COST ESTIMATE

The budget for different activities required to be carried out for mitigation and prevention of dam break hazard exclusively from the dam is given in Table-17.3.

**TABLE-17.3**

**Cost Estimate for Implementing DMP**

<b>S. No.</b>	<b>Particular</b>	<b>Cost (Rs. million)</b>
1.	Installation of alert system in control room and at Karanprayag	1.0
2.	Setting up of communication system between u/s and d/s of project	2.0
3.	Setting up of communication system between dam and d/s settlements	5.0
4.	Public information system	0.5
5.	Setting up of a seismic observatory at dam site	5.0
6.	Flood forecasting	16.0
7.	Training and miscellaneous	0.5
	<b>Total</b>	<b>30.0</b>